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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

CONTENTS SHEET NO.

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20	LAB RESULTS

SITE PHOTOGRAPHS

DESCRIPTION

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY _IREDELL

PROJECT DESCRIPTION REPLACE BRIDGE NO. 189 ON SR 1892 (IENNINGS ROAD) OVER SOUTH YADKIN RIVER

STATE PROJECT REFERENCE NO. 21 B-5846

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1999) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLORDING TO CLIMATIC CONDITIONS INCLORDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

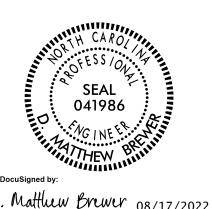
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

CG2 EXPLORATION
S.N. PATTERSON, G.I.T.
INVESTIGATED BY CG2, PLLC
DRAWN BY S. N. PATTERSON, G.I.T.
CHECKED BY M. BREWER, P.E.



SUBMITTED BY <u>CG2</u>, PLLC

DATE <u>MAY</u> 2022



D. Matthew Brewer 08/17/2022 -386129C0A4C1462 SIGNATURE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLICIALT POWER AUGUER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DISBG). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOSITURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT ACTORS SUCH	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN Ø.I FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDGED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	THE ANGULARITY OR ROUNDIESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH ID ODES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY	CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK (CR) ROCK	SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE	COASTAL PLAIN COASTA	OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*18 58 MX GRANULAR CILL MUCK *48 38 MX 58 MX 51 MN SOILS COILS COILS COILS		(CP)	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
**289 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN	ORGANIC MATERIAL GRANULAR SILT - CLAY SOLS SOLS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	$\underline{ ext{DP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LL 40 MX 41 MN LITTLE OR PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR HIGHL	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CROUP INDEX		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. DEEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELOSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITING ALONG CLOSELY SPACED PARALLEL PLANES.
UF PRAURY MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS GEN PATHIC FAIR TO	STATIC WATER LEVEL AFTER 24 HOURS \[\sum_{PW} \] PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
EXCELLENT TO GOOD	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS COMPACTNESS OR RANGE OF UNCONFINED COMPACTNESS OR RESPONSIBILITIES CONTROLLERS CONTROLLERS OR RESPONSIBILITIES CONTROLLERS CONT		SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SYT REFUSAL.	FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTI (TONS/FT ²) GENERALLY VERY LOOSE < 4 TO 100	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES SOIL SYMBOL SOIL SYMBOL SOIL SYMBOL STRUCTURES SERVING TEST BORDING ✓ SLOPE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELOSPARS ARE KAOLINIZED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
COSE	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER TEST	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY DENSE	INFERRED SOIL BOUNDARY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. OF AN INTERVENING IMPERVIOUS STRATUM. DESCRIPTION (NEC. SCI.,
SILT-CLAY	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE PIEZOMETER INSTALLATION SPT N-VALUE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK, ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PAREN'
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE ACCEPTABLE, BUT NOT TO BE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
DPENING (MM)	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.) GRAIN MM 305 75 2.0 0.25 0.05 0.005	ABBREVIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 - DRY UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGEO 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	ON CSE. COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TO TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS ω - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FINGERNAIL. FRACTURE SPACING BEDDING	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. BENCH MARK: EXISTING END BENT I: BRIDGE SEAT LEFT OF -EL-
(P)) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 789.86 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID: AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS X AUTOMATIC MANUAL	WIDE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	G* CONTINUOUS FLIGHT AUGER CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED 0.008 FEET INDURATION	F.I.A.D. = FILLED IMMEDIATELY AFTER DRILLING ROADWAY DESIGN AND SURVEY INFORMATION DATED 10/06/2021
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	PROVIDED BY TGS ENGINEERS. NO AVAILABLE BENCHMARKS WERE SET OR LOCATABLE AT THE
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	TUNGCARBIDE INSERTS HAND TOOLS:	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	NO AVAILABLE BENCHMARKS WERE SET OR LOCATABLE AT THE PROJECT SITE DURING THE INVESTIGATION. A TBM WAS UTILIZED WITH THE ELEVATION AS REFERENCED ON THE SURVEY DGN.
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING X W/ ADVANCER POST HOLE DIGGER HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	X DIEDRICH D-50	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X CORE BIT VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	

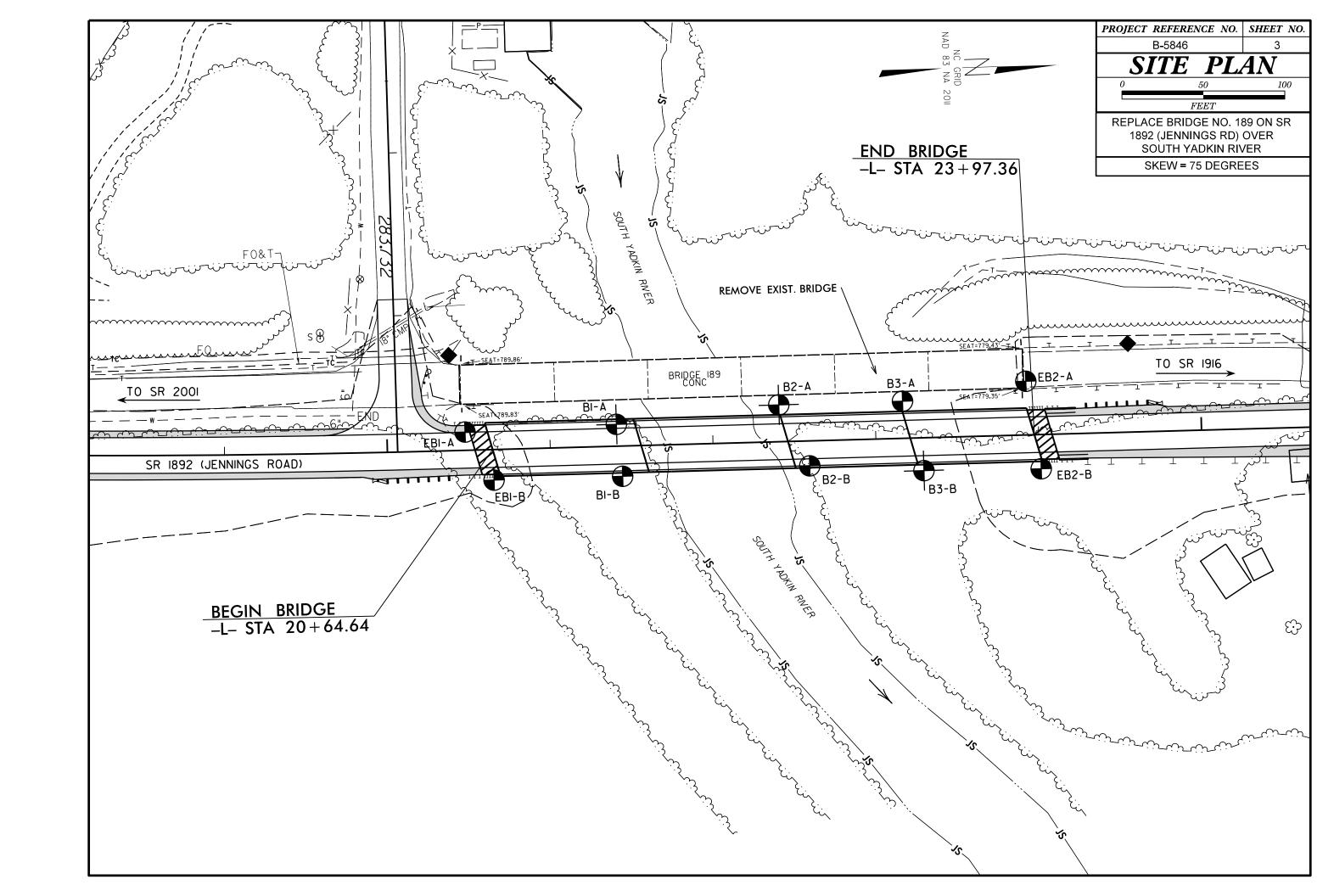
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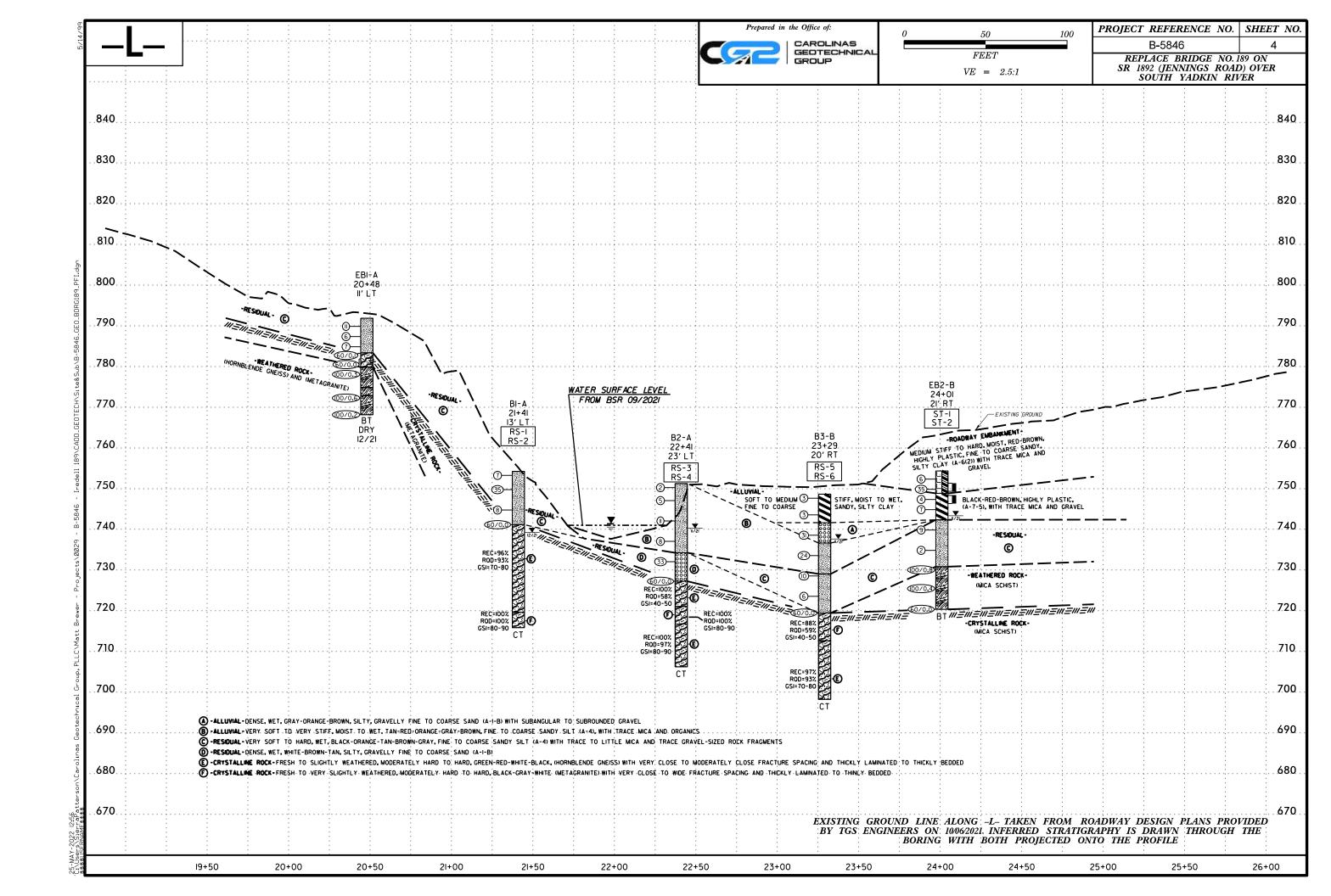
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

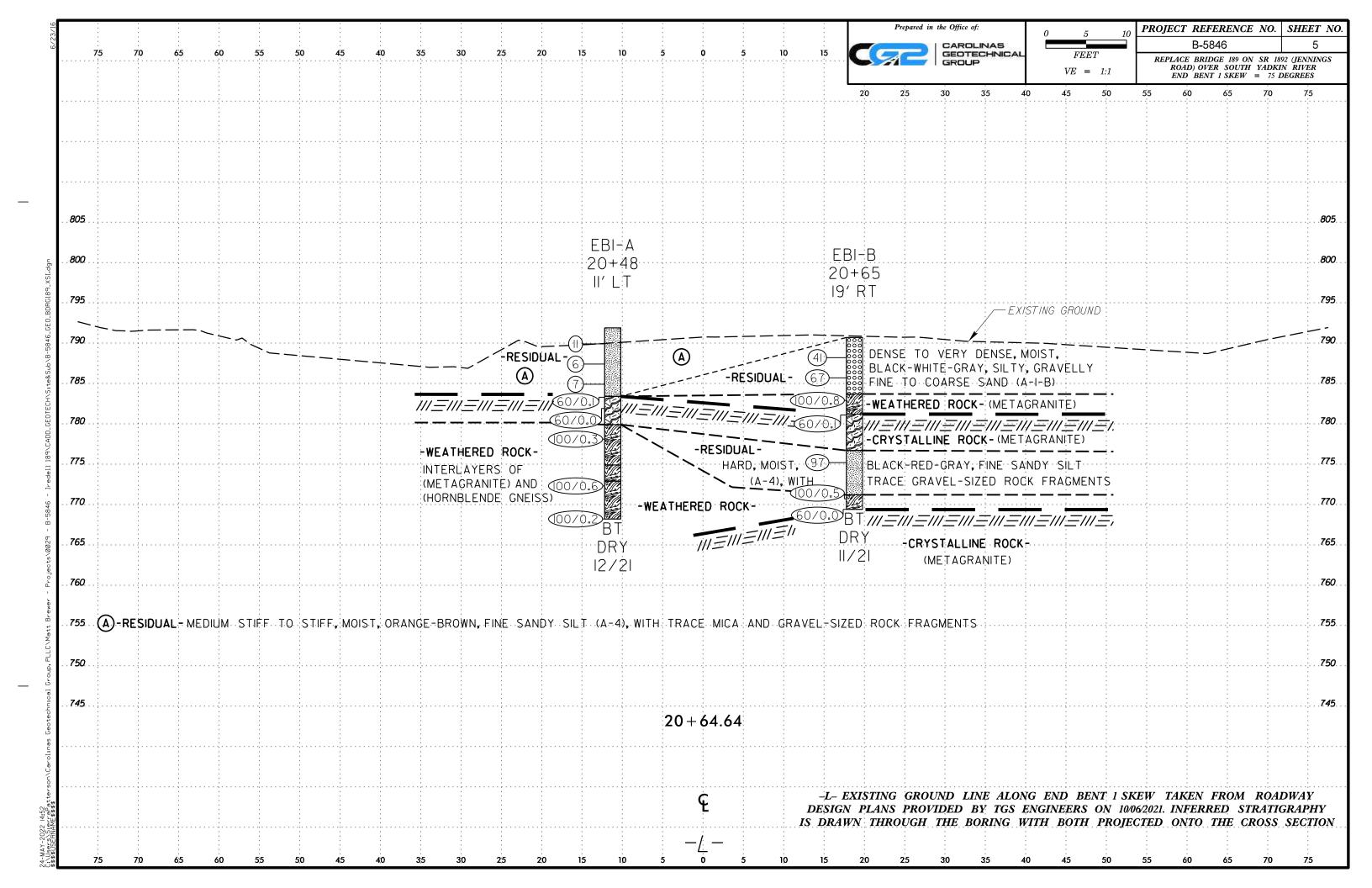
SUBSURFACE INVESTIGATION

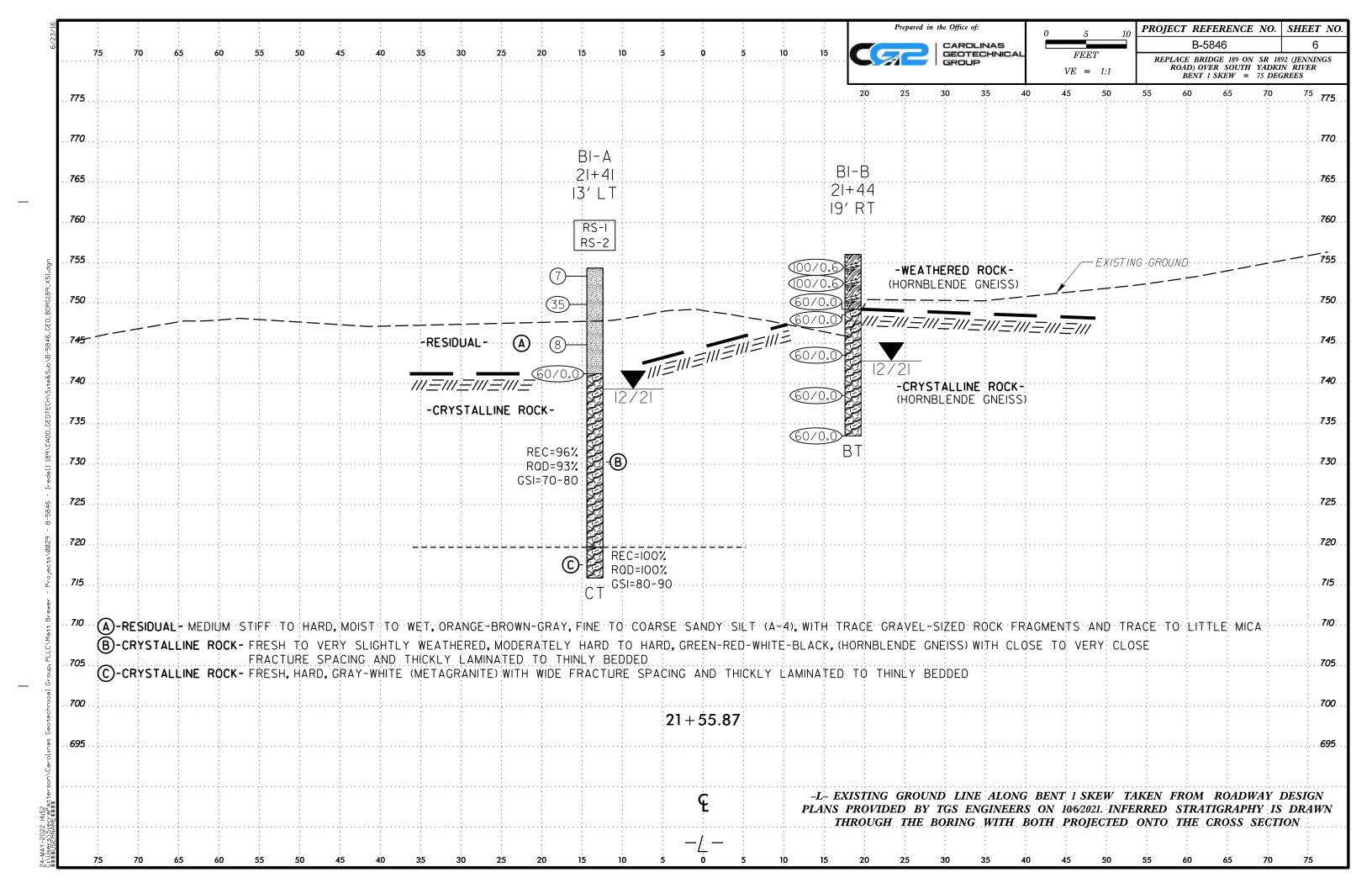
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

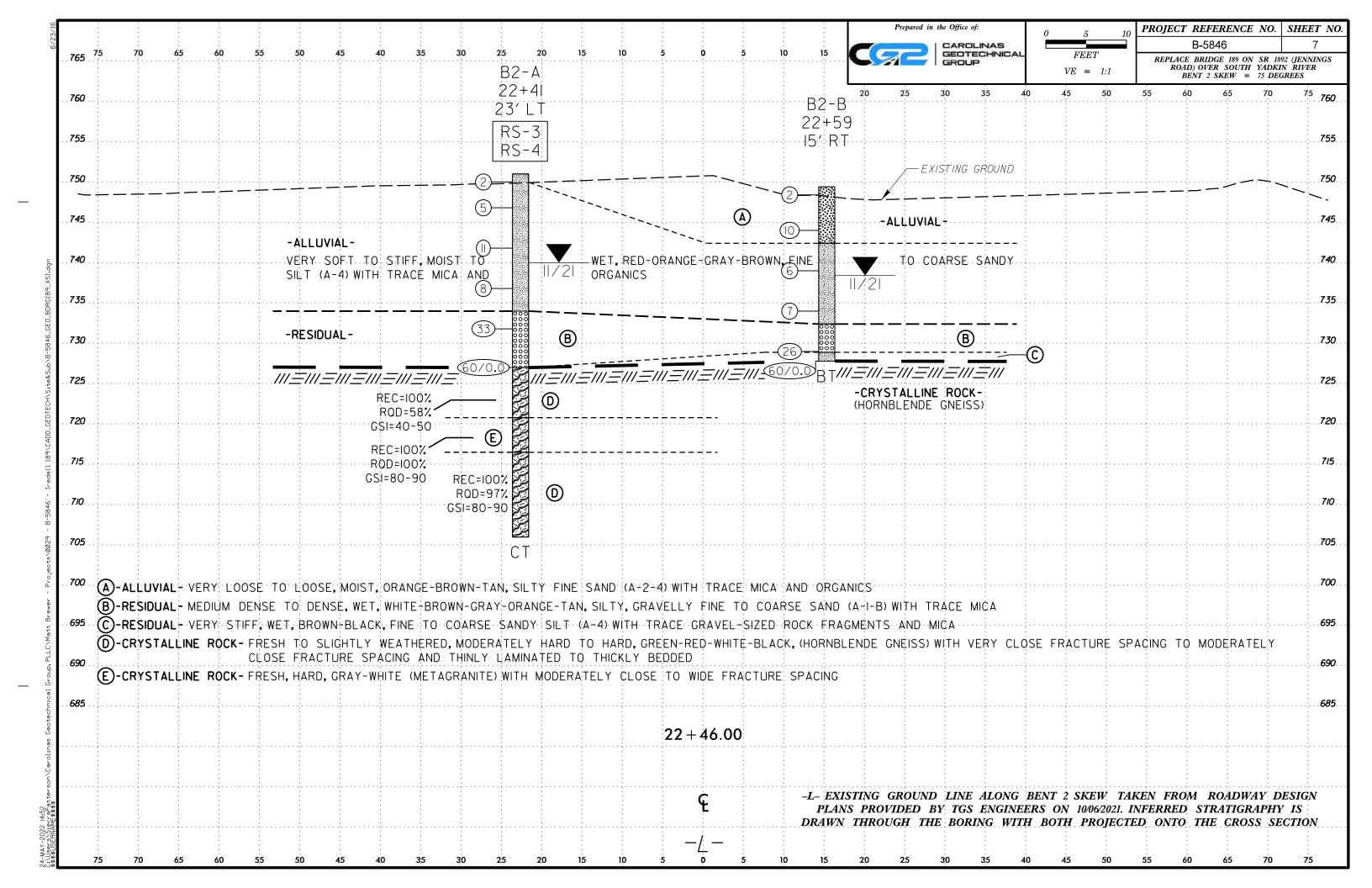
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Joint		FROM AA	SHTO ĹRFD	BRID	AAL STRENGTH INDEX (GSI) TABLES GE DESIGN SPECIFICATIONS		D 1	M /M		2000
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000) From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis. STRUCTURE	VERY GOOD Very rough, fresh unweathered surfaces	Constrained Surfaces Surfaces	1	VERY POOR Slickensided, highly weathered surfaces with soft clay coatings or fillings	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000) From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis. COMPOSITION AND STRUCTURE	VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slicken- sided or highly weathered surfaces with soft clay coatings or fillings
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90 80		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70 60	A			
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	70 CKING OF	60 50			B. Sand- stone with stone and siltstone in similar siltstone siltstone siltstone amounts Solution siltstone or silty shale with sand- stone layers shale with sandstone layers		50 B 40	c [D E	
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	N	40	30		C.D.E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.			30	F 20	
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECKEASING		20	10	G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.			(10
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	V N/A	N/A	///	,10 /	─────────────────────────────────────			/ /		DATE: 8-19-10

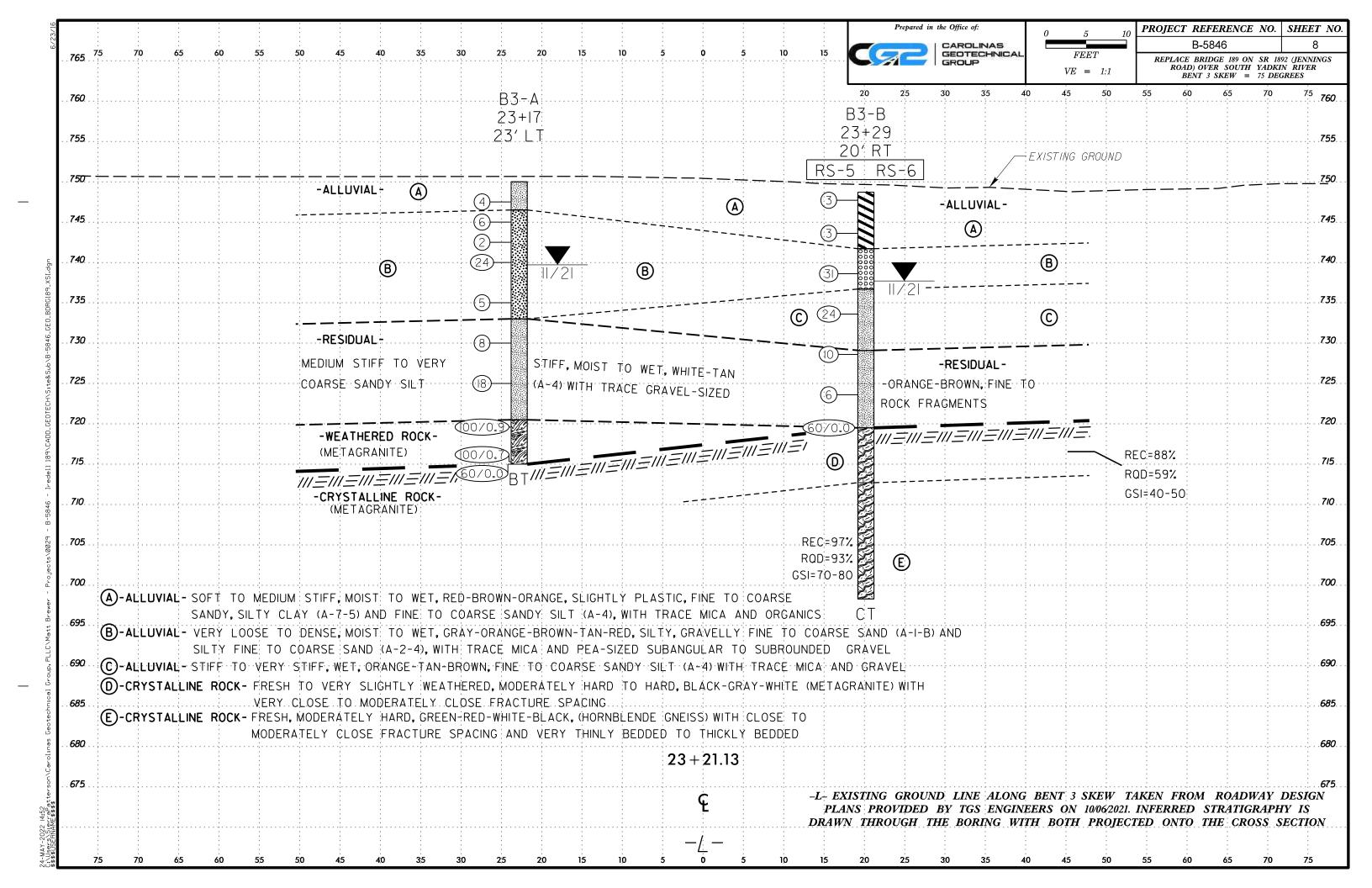


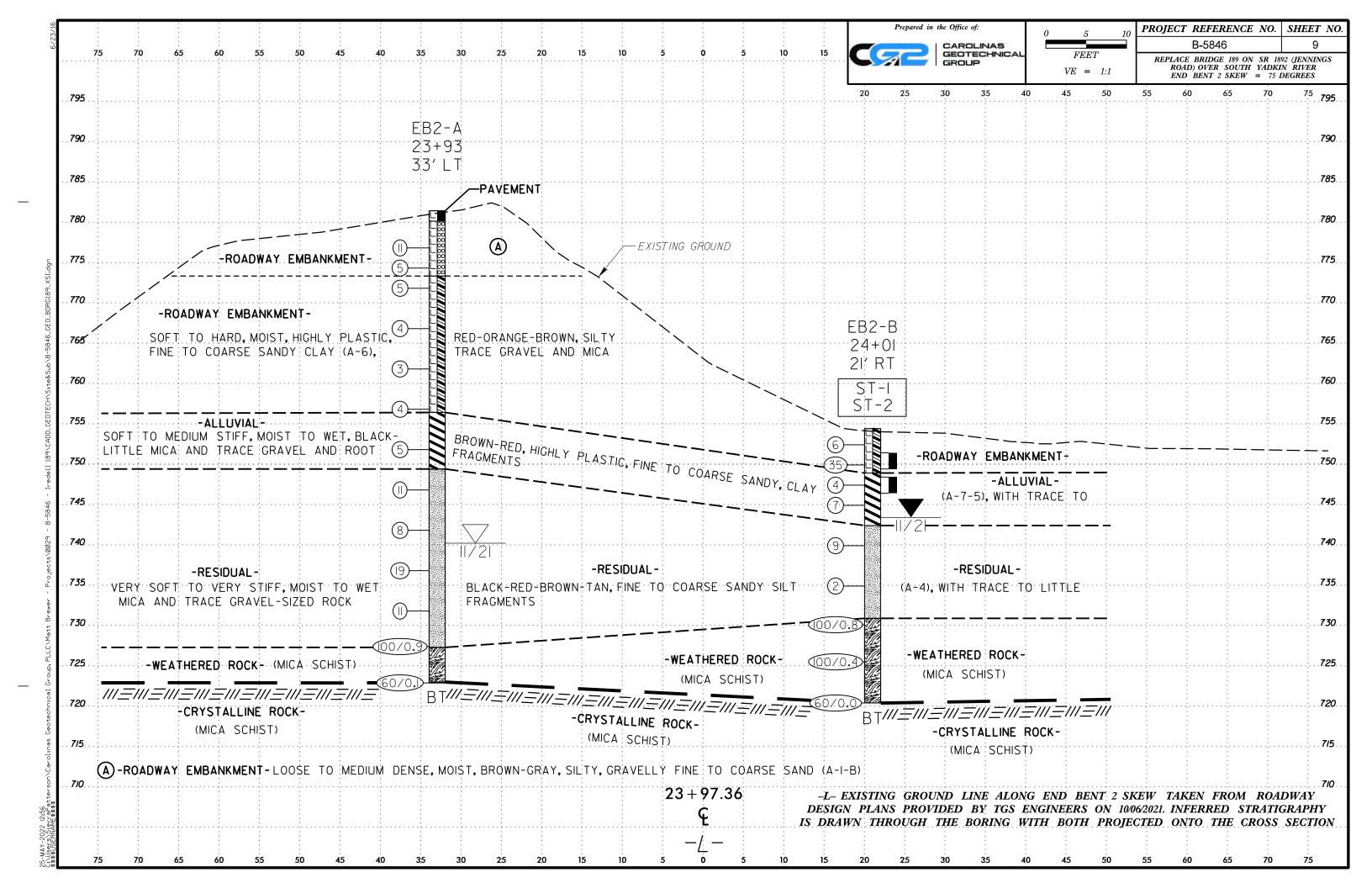


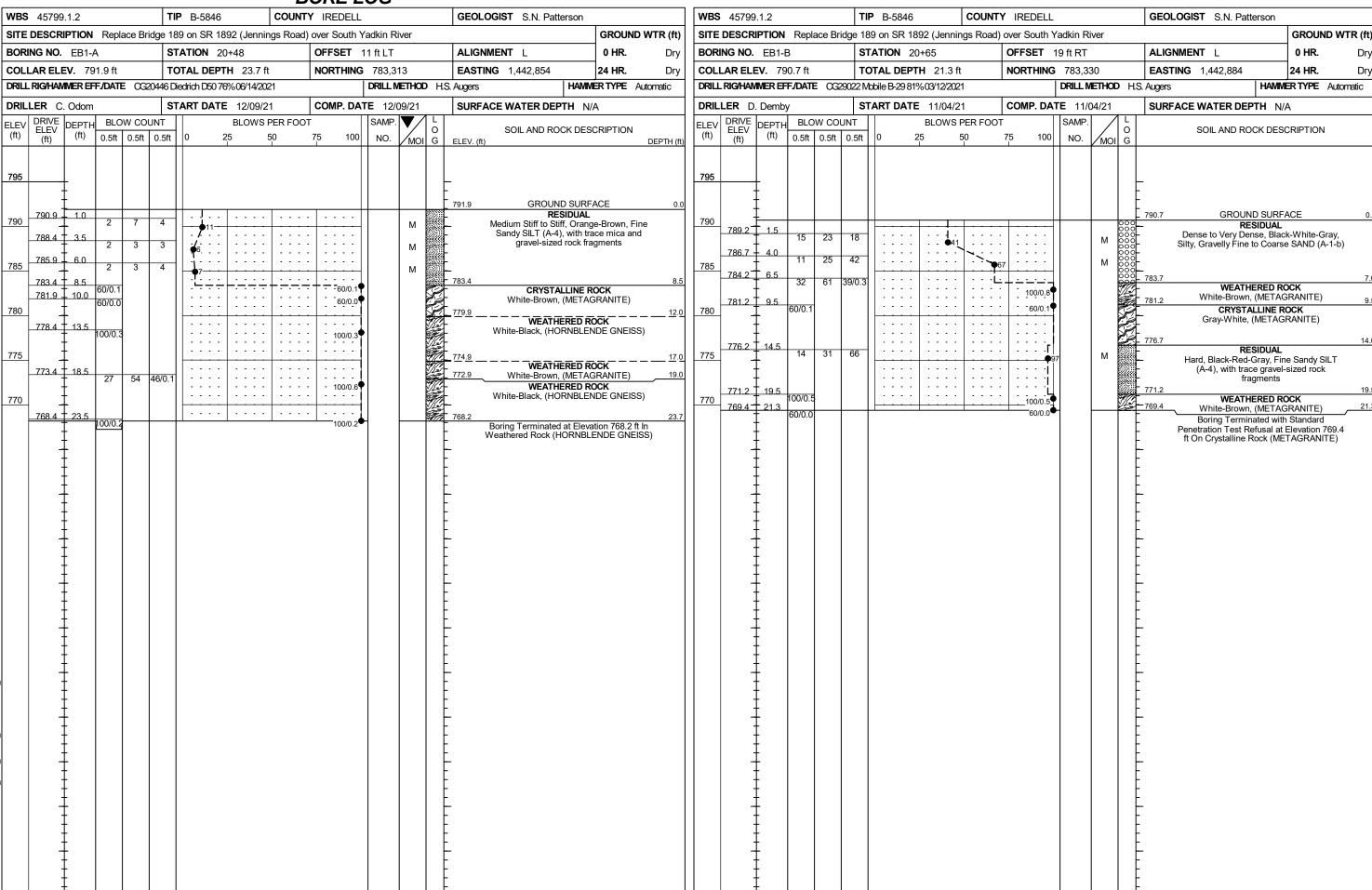












	BORE I	LOG				CORE LOG
WBS 45799.1.2	TIP B-5846 COUNTY IREDEL	L GEOLOGIST S.N. Patterson	WBS 45	5799.1.2	TIP B-5846 C0	OUNTY IREDELL GEOLOG
SITE DESCRIPTION Replace Bridge	e 189 on SR 1892 (Jennings Road) over South	Yadkin River	GROUND WTR (ft) SITE DES	SCRIPTION Replace Bridg	ge 189 on SR 1892 (Jennings	Road) over South Yadkin River
BORING NO. B1-A	STATION 21+41 OFFSET	13 ft LT ALIGNMENT L	0 HR. N/A BORING	NO . B1-A	STATION 21+41	OFFSET 13 ft LT ALIGNME
COLLAR ELEV. 754.3 ft		G 783,406 EASTING 1,442,852		RELEV. 754.3 ft	TOTAL DEPTH 38.4 ft	NORTHING 783,406 EASTING
DRILL RIG/HAMMER EFF./DATE CG2044					146 Diedrich D50 76% 06/14/2021	DRILL METHOD NW Casing w/ Ad
		ATE 12/08/21 SURFACE WATER DEPTH N/	<u> </u>	R C. Odom	START DATE 12/07/21	COMP. DATE 12/08/21 SURFACE
ELEV (ft) DRIVE ELEV (ft) DEPTH BLOW COUNT (ft) 0.5ft 0.5ft 0.5		SAMP. L O SOIL AND ROCK DES			TOTAL RUN 25.3 ft	RATA I I
	0.10	0 NO. MOI G ELEV. (ft)	(ft) ELEV (ft) (ft)	UN LEV (ft) RUN RATE (Min/ft)	REC. ROD SAMP. REC.	
755 0.0 2 2 5	5	-754.3 GROUND SURF	ACE 0.0 741.2 740 74	31.2 13.1 1.5 N=60/0.0 99.7 14.6 5.0 6:56/1.5	0 (1.1) (1.0) (20.6) 73% 67% 96%	Begin Col (20.0) 741.2 CRYST. 93% Fresh to Very Slightly Wea
		M RESIDUAL Medium Stiff to Hard, Orang Fine to Coarse Sandy SILT	e-Brown-Gray,	5.0 4:01/1.0 4:05/1.0	73% 67% (4.5) (4.2) 90% 84%	Green-Red-White-Black, (HORN
750 750.8 + 3.5 7 14 2	35	M gravel-sized rock fragmen	s and trace to	+ I I 3:48/1.0) RS-1	Close Fracture Spacing and
‡			735 73.	34.7 19.6 4:23/1.0 + 5.0 3:14/1.0	0 (4.9) (4.9) 0 98% 98%	RS-1: Unit We
745.8 + 8.5 2 2 6				+ 3:11/1.0)	Unconfined Compressive
	1		730 72	29.7 24.6 3:35/1.0 3:39/1.0 5.1 4:05/1.0	/	RS-2 Unit We Unconfined Compressive
741.2 13.1 60/0.0		741.2	13.1	3:59/1.0 4:16/1.0) 100% 96%	Oncommed Compressive
740		Green-Red-White-Black, (h		24.6 29.7 3:53/1.0 4:36/1.1		J. E.
				- 5.2 4:54/1.0 + 4:48/1.0) (5.2) (5.2)) 100% 100%	
735		RS-1 REC=96% RQD=93% GSI=70-80	720	+ 4:47/1.0 2:41/1.0	3	719.7
‡		GSI-70-60	720 71!	9.4 + 34.9 2:54/1.2	(3.8)	(3.8) Fresh, Hard, Gray-White (META)
730		RS-2	71	15 O T 20 A 115:51/1.0	0	and Thickly Lami
				42:06/0.5	5	Boring Terminated at Elev (MET
				Ī		MET (MET
725				<u> </u>		
				‡		
720		719.7	34.6	‡		
‡		Gray-White, (METAG	RANITE)	‡		
		715.9 RQD=100%	38.4	‡		
		GSI=80-90 Boring Terminated at Eleva Crystalline Rock (META	tion 715.9 ft In	‡		-
		Crystalline Rock (META	GRANITE)	‡		
				<u> </u>		1 E
				Ţ		
+			1/22	‡		
1 5/24			T 5/24/22	‡		-
T.GD.T.			DOT.GDT	‡		
				‡		-
		[N C	‡		
0-III			GTM.GPJ	<u> </u>		
			<u> </u> <u> </u> <u> </u>	Ŧ		1 E
BRDG + + +			BRDG	‡		
G T T T T T T T T T T T T T T T T T T T		‡	GEO	‡		
-5846			B-5846_GEO	‡		
H H				‡		
Door Helphone			INGL	‡		
BORED +)RE S	‡		
			ICDOT CORE SINGLE	‡		
			CDC	‡		

GEOTECHNICAL BORING REPORT

									C	Ol	RE L	OG						
WBS	45799	.1.2			TIP	B-584	6	С	OUNT	Y II	REDELL			GEOLOGIST S.N. Pat	terson			7
SITE	DESCRI	PTION	Repla	ace Bridge	e 189 d	on SR	1892 (Jer	nnings	Road)	ove	r South Y	adkin River		•		GROU	ND WTR (ft))
BORI	NG NO.	B1-A			STAT	TION	21+41			OF	FSET	13 ft LT		ALIGNMENT L		0 HR.	N/A	۱ ا
COLL	AR ELE	V . 75	4.3 ft		TOTA	AL DE	PTH 38.	4 ft		NC	RTHING	783,406		EASTING 1,442,852		24 HR.	15.0	ر
DRILL	RIG/HAM	MER EF	EFF/DATE CG20446 Diedrich D50 76%06/14/2021 DRILL METHOD NW Casing w/ Advancer HAMMER TYPE Automatic									1						
DRILI	LER C.	Odom			STAF	RT DA	TE 12/0	7/21		СС	MP. DA	ГЕ 12/08/21		SURFACE WATER DE	TH N/	A		1
CORE	E SIZE	NQ			TOTA	AL RUI	N 25.3 f	t						•				1
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	ATA RQD (ft) %	L O G	ELEV. (1	7)	DE	DESCRIPTION AND REMARKS DEPTH				
741.2	. ,			,	70	70		70	7.0					Begin Coring @ 13.1 ft			<i>32.</i> (.	
740	741.2 - 739.7 -	- 13.1 - 14.6	1.5	N=60/0.0 6:56/1.5	(1.1) 73% /	(1.0) (67%)		(20.6) 96%	(20.0) 93%		741.2	Fresh to V	lony S	CRYSTALLINE ROCK Slightly Weathered, Modera	toly Hard	to Hard	13.	.1
	- -	- - -	5.0	4:01/1.0 4:05/1.0 4:21/1.0	(4.5) 90%	(4.2) 84%	RS-1	3070	9370		}	Green-Red-Whi	te-Bl	Slack, (HORNBLENDE GNE Spacing and Thickly Laminat	SS), with	Very Clos		
735	734.7	19.6	5.0	3:48/1.0 4:23/1.0	(4.0)	(4.0)	110-1	1			-			RS-1: 17.4 - 17.9' Unit Weight: 176.5 pcf				
	-	-	5.0	3:14/1.0 3:45/1.0	(4.9) 98%	(4.9) 98%					<u> </u>	Unconfin	ned C	Compressive Strength: 8,03) psi (1,1	56 ksf)		
730		-		3:11/1.0 3:35/1.0			RS-2	į			-			RS-2: 22.2 - 22.8'				
730	729.7_	<u>24.6</u>	5.1	3:39/1.0 4:05/1.0	(5.1)	(4.9)					-	Unconfine	ed C	Unit Weight: 178.1 pcf Compressive Strength: 11,11	0 psi (1,6	600 ksf)		
	-	-		3:59/1.0 4:16/1.0	100%	96%					ļ			GSI=70-80				
725	724.6 -	- - 29.7		3:53/1.0 4:36/1.1							Ļ							
	-	-	5.2	4:54/1.0 4:48/1.0	(5.2) 100%	(5.2) 100%					ļ.							
720	-	-		4:47/1.0 2:41/1.0							-							
120	719.4 -	- 34.9 -	3.5	2:54/1.2 5:55/1.0	(3.5)	(3.5)		(3.8)	(3.8) 100%		719.7 _			hite (METAGRANITE), with		acture Spa	34. acing	.6
	715.9	38.4		9:58/1.0 15:51/1.0				100%	100%		715.9	a	and T	Thickly Laminated to Thinly	Bedded		20	1
	715.9	_ 30.4		42:06/0.5						رزاهي	_ / 15.9	Boring Te	rmin	GSI=80-90 nated at Elevation 715.9 ft In	Crystallir	ne Rock		4
	-	- -										209 . 0		(METAGRANITE)	o. jotaiii.			
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Replace Bridge 189 on SR 1892 (Jennings Road) over South Yadkin River, Iredell County, NC Rock Core Photographs Boring: B1-A

13.1 to 38.4 Feet

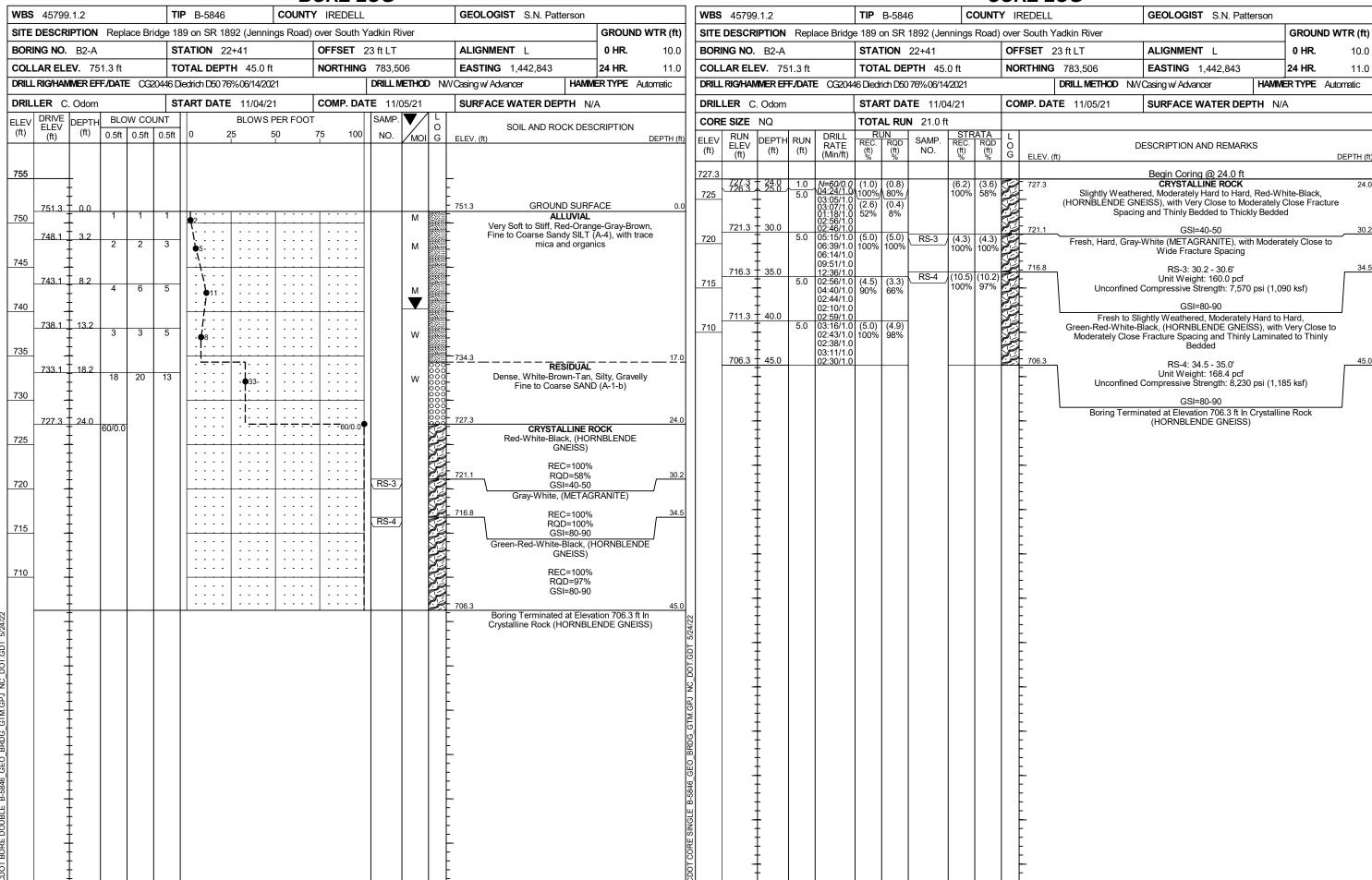




										D	Un	KE L	UG				
WBS	45799	.1.2			TI	P B-5	846		С	OUNT	Y IRI	EDELL				GEOLOGIST S.N. Patterson	
SITE	DESCR	IPTION	Repl	ace Bri	dge 18	39 on S	R 189	92 (Jen	nings	Road)	over :	South \	⁄adkin	River		GROUND W	TR (ft)
BORI	NG NO.	B1-B			S	TATION	l 21	+44			OFF	SET	19 ft R	Т		ALIGNMENT L 0 HR.	N/A
COLI	AR ELE	EV . 75	6.0 ft		Т	OTAL D	DEPTI	H 22.5	ft		NOR	THING	783	409		EASTING 1,442,884 24 HR .	13.2
	.RIG/HAIV			E CG2											D N	W Casing w/ SPT HAMMER TYPE Autor	matic
	LER C					TART D					CON	IP. DA				SURFACE WATER DEPTH N/A	
		DEPTH		w col						R FOO			SAM		/ L	CONTACE WATER BEITH N/A	
ELEV (ft)	DRIVE ELEV (ft)	(ft)	0.5ft			0	2		50		75 	100	NO	1 '/	0		EPTH (ft
760	- - -	- - -															
7	-					<u> </u>									V(1)20	756.0 GROUND SURFACE	0.0
755	755.0_	1.0	50	50/0.1		 			_		. -	100/0.6	•			WEATHERED ROCK Brown-Black, (HORNBLENDE GNEISS)	
	753.0	3.0	87	13/0.1		::	: :		-		. .	100/0.6	,				
750	-	-							-		. -						
	749.2 748.5	6.8 7.5	60/0.0						-			60/0.0	2			749.2 CRYSTALLINE ROCK	6.
	-	-	60/0.0	1					:		. -	- 60/0.0 				Gray-Black, (HORNBLENDE GNEISS)	
745	-	_							-							_	
	743.5	12.5	60/0.0	-		: :	: :				. .	60/0.0	•				
	-	-	00/0.0						-		. -					_	
740	_	F							-		 -					_	
	738.5	17.5	60/0.0	1							: :	60/0.0	•				
	-	_				: :					. -	: : :				<u> </u>	
735	733.5					 			-		+-					733.5	22.
	/33.5	22.5	60/0.0									60/0.0	۲		نراهج	Boring Terminated with Standard	
	-	<u> </u>														Penetration Test Refusal at Elevation 733.5 ft In Crystalline Rock (HORNBLENDE	
	-	ŀ														GNEISS)	
	-	-														Notes:	
	-	-														Switch to PDC Bit at 7.5 ft	
	-	<u> </u>															
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SHEET 13

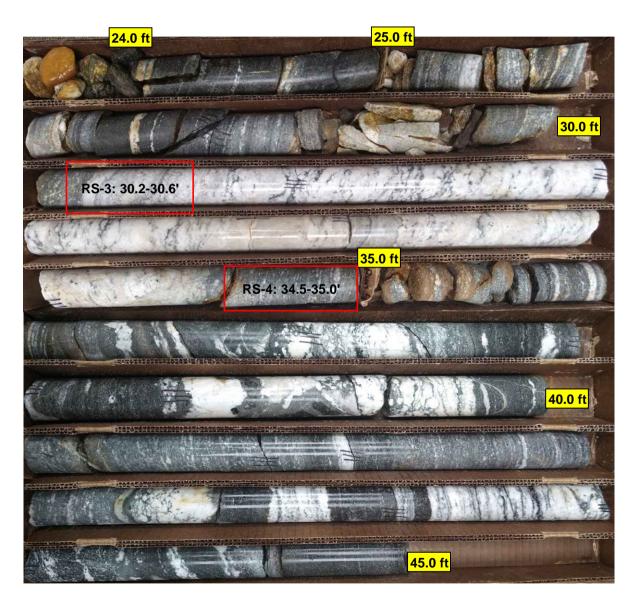
GEOTECHNICAL BORING REPORT CORE LOG

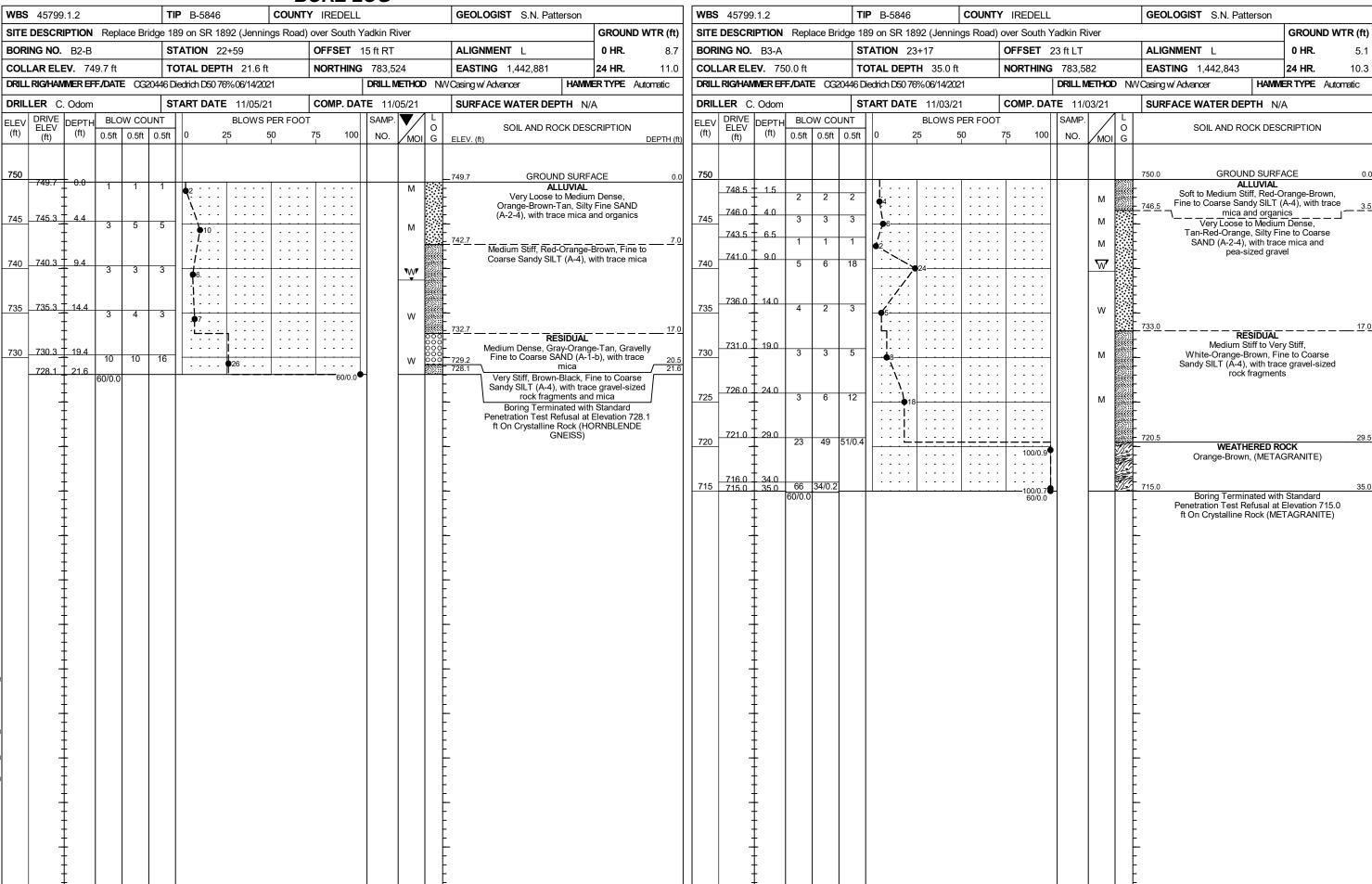




Replace Bridge 189 on SR 1892 (Jennings Road) over South Yadkin River, Iredell County, NC Rock Core Photographs Boring: B2-A

24.0 to 45.0 Feet





GEOTECHNICAL BORING REPORT CORE LOG

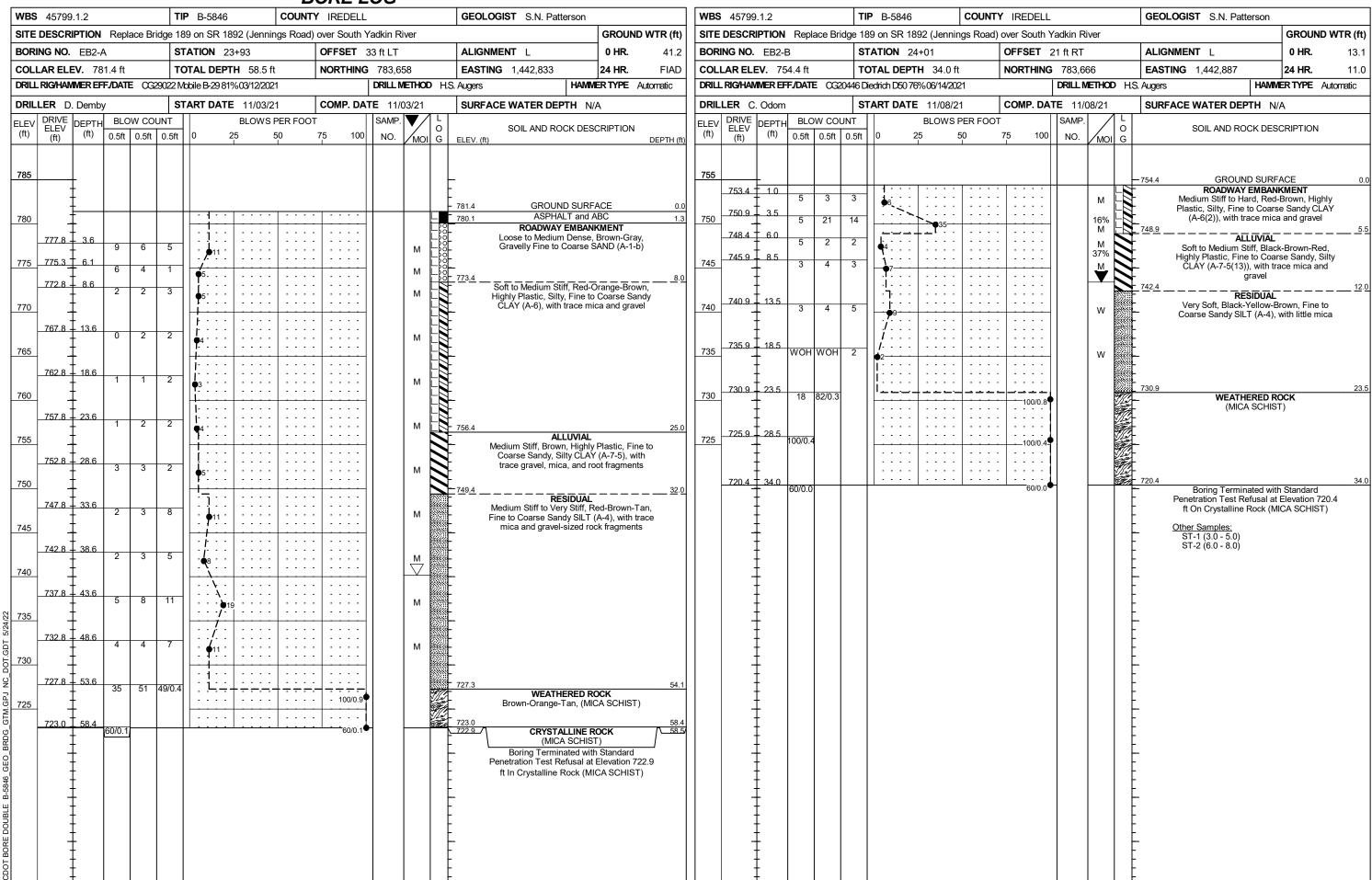
		BORE LOG		_	CORE L	OG	
WBS 45799.1.2	TIP B-5846 CO	UNTY IREDELL	GEOLOGIST S.N. Patterson	WBS 45799.1.2	TIP B-5846 COUNTY IREDELL	GEOLOGIST S.N. Patterson	
SITE DESCRIPTION Replace Br	<u> </u>	load) over South Yadkin River	GROUND WTR (f	SITE DESCRIPTION Replace Brid	ge 189 on SR 1892 (Jennings Road) over South Ya	adkin River	GROUND WTR
BORING NO. B3-B	STATION 23+29	OFFSET 20 ft RT	ALIGNMENT L 0 HR. N/.	BORING NO. B3-B	STATION 23+29 OFFSET 2	0 ft RT ALIGNMENT L	0 HR.
COLLAR ELEV. 748.7 ft	TOTAL DEPTH 50.4 ft	NORTHING 783,594	EASTING 1,442,886 24 HR. 11.	COLLAR ELEV. 748.7 ft	TOTAL DEPTH 50.4 ft NORTHING	783,594 EASTING 1,442,886	24 HR . 1
DRILL RIG/HAMMER EFF./DATE CG2			W Casing w/ Advancer HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF/DATE CG20	146 Diedrich D50 76% 06/14/2021	DRILL METHOD NW Casing w/ Advancer HAM	MERTYPE Automat
DRILLER C. Odom	START DATE 11/08/21	COMP. DATE 11/08/21	SURFACE WATER DEPTH N/A	DRILLER C. Odom	START DATE 11/08/21 COMP. DAT	SURFACE WATER DEPTH	I/A
ELEV DRIVE ELEV (ft) DEPTH BLOW CO		75 400	SOIL AND ROCK DESCRIPTION	CORE SIZE NQ	TOTAL RUN 21.2 ft		
(It) (ft) (It) 0.5ft 0.5ft	0.511 0 25 50	75 100 NO. MOI G	ELEV. (ft) DEPTH	ELEV RUN DEPTH RUN RATE (ft) (ft) (ft) (ft) (ft)	RUN SAMP. REC. RQD O (ft) (ft) (ft) % % G ELEV. (ft)	DESCRIPTION AND REMARKS	
750					% % G ELEV. (ft	,	DEP
750 748.7 - 0.0				719.5 719.5 29.2 1.2 N=60/0. 07:03/14.6 07:03/14	(6.0) (4.0) 719.5 2 (100% 83% 88% 59% 719.5	Begin Coring @ 29.2 ft CRYSTALLINE ROCK	
	 		- ALLUVIAL Soft, Red-Brown, Highly Plastic, Fine to		2/100% 83% / 88% 59% 59% 59% 59% 59% 60%	Fresh to Very Slightly Weathered, Moderately Har Black-Gray-White (METAGRANITE), with Very Close to	d to Hard, Moderately Close
745 744.6 4.1	<u> </u>		- Coarse Sandy, Silty CLAY (A-7-5), with trace mica	715	0 85% 48%	Fracture Spacing	
	1 •3	::: :::: w		.0 09:45/0. - 5.2 04:09/1. - 02:45/1. 03:17/1.	(4.8) (4.1) RS-5 (13.9) (13.4) 712.7	RS-5: 35.3 - 35.6' Unit Weight: 162.7 pcf	Г
740		000	Dense, Gray-Orange-Brown, Silty, Gravelly Fine to Coarse SAND (A-1-b), with	. <u>0</u> 710	92% 79% (13.9) (13.4) 97% 93%	Unconfined Compressive Štrength: 14,800 psi (2	2,130 ksf)
739.6 9.1 7 12			- subangular to subrounded gravel	708.5 + 40.2 04:31/1.	2 RS-6	GSI=40-50 CRYSTALLINE ROCK	
<u> </u>	/	000	736.7 Stiff to Very Stiff, Orange-Tan-Brown, Fine	T 705 +	0 (5.0) (4.8) 100% 96%	Fresh, Moderately Hard, Green-Red-White-Black, (H- GNEISS), with Close to Moderately Close Fracture Sp Thinly Bedded to Thickly Bedded	ORNBLENDE acing and Very
735 734.6 14.1			to Coarse Sandy SILT (A-4), with trace mica and gravel	703.5 + 45.2 03:42/1. 03:57/1.		Thinly Bedded to Thickly Bedded	
	24			+ 5.2 4:20/1.0 + 4:34/1.0		RS-6: 39.3 - 39.8' Unit Weight: 190.6 pcf	
730 729.6 19.1			729.1	4:20/1.0 3:58/1.0		Unconfined Compressive Strength: 9,700 psi (1	397 ksf)
4 4	6	· · · · · · ·	RESIDUAL	698.3 + 50.4 5:02/1.2	698.3	GSI=70-80 Boring Terminated at Elevation 698.3 ft In Crystal	line Rock
725	:/:::: ::::: ::		Medium Stiff to Stiff, Orange-Tan-Brown-Gray, Fine to Coarse			(HORNBLENDE GNEISS)	o r took
	4		Sandy SILT (A-4), with trace gravel-sized rock fragments				
			-				
720 719.5 29.2		· · · · · · · ·	├ ├─ 719.5 29	_2			
60/0.0			CRYSTALLINE ROCK Black-Gray-White, (METAGRANITE)	7 ‡			
715			REC=88%				
/ 10			RQD=59% GSI=40-50]			
		RS-5	712.7 GS1=40-50 36	····			
710 +			Green-Red-White-Black, (HORNBLENDE GNEISS)				
		RS-6	REC=97%				
705			RQD=93% GSI=70-80				
\neg \exists \mid \mid							
			<u> </u>				
700			608.3	\$4155 4 +			
+ + +			Boring Terminated at Elevation 698.3 ft In Crystalline Rock (HORNBLENDE GNEISS)	<u>.4 8 </u>			
			L Grystaillie Rock (HORNDLENDE GNEISS)				
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Replace Bridge 189 on SR 1892 (Jennings Road) over South Yadkin River, Iredell County, NC Rock Core Photographs Boring: B3-B

29.2 to 50.4 Feet





PROJECT REFERENCE NO.	SHEET NO.
B-5846	20
LAB RESU	<i>ILTS</i>

	ROCK TEST RESULTS												
SAMPLE NO.	BORING	STATION	OFFSET	DEPTH INTERVAL	ROCK TYPE	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH						
RS-1	B1–A	21+41 -L-	13' LT	17.4 - 17.9'	HORNBLENDE GNEISS	176.5	8,030 psi/1,156 ksf						
RS–2	B1–A	21+41 -L-	13' LT	22.2 - 22.8'	HORNBLENDE GNEISS	178.1	11,110 psi/1,600 ksf						
RS–3	B2-A	22+41 -L-	$23^{\circ}LT$	30.2 - 30.6'	METAGRANITE	160.0	7,570 psi/1,090 ksf						
RS–4	B2-A	22+41 -L-	23' LT	34.5 - 35.0'	HORNBLENDE GNEISS	168.4	8,230 psi/1,185 ksf						
RS–5	В3–В	23+29 -L-	20' RT	35.3 - 35.6'	METAGRANITE	162.7	14,800 psi/2,130 ksf						
RS-6	В3-В	23+29 -L-	20' RT	39.3 - 39.8'	HORNBLENDE GNEISS	190.6	9,700 psi/1,397 ksf						

LAB TESTING PERFORMED BY NCDOT LAB CERT NO. 117-1104

SOIL TEST RESULTS															
SAMPLE	OFFSET	STATION	DEPTH	AASHTO		D 1		% BY WEIGHT			% PASSING (SIEVES)			%	%
NO.	OFFSEI	STATION	INTERVAL	CLASS.	L.L.	P.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
ST-1	21' RT	24+01 -L-	3.0 - 5.0'	A-6(2)	31	13	25.2	31.3	15.2	28.2	88.1	76.9	40.8	16.0	_
ST-2	21' RT	24+01 -L-	6.0 - 8.0'	A-7-5(13)	71	28	18.7	21.8	12.8	46.7	82.0	67.1	52.0	36.8	1

LAB TESTING PERFORMED BY NCDOT LAB CERT NO. 134-04

PROJECT REFERENCE NO.	SHEET NO.			
B-5846	21			
SITE PHO	TOS			



Photo #1: End Bent 2 looking south (downstation)



Photo #2: End Bent 1 looking north/northeast (upstation)



Photo #3: Right side of existing bridge looking north/northwest (upstation)